CIRCLE K SOUTHWEST PROTOTYPE 4480 – 08/13/2013

A. GENERAL

1. GENERAL CONTRACTORS SHALL VERIFY ALL GRADES, DIMENSIONS AND CONDITIONS AT THE JOB SITE WITH COMPLETE SET OF DRAWINGS PRIOR TO BIDDING AND COMMENCING CONSTRUCTION. THE ARCHITECT OR ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCIES OR INCONSISTENCIES.
2. ALL MATERIALS AND WORK PERFORMED SHALL CONFORM TO THE LATEST REQUIREMENTS OF THE 2012 INTERNATIONAL BUILDING CODE AND ANY OTHER APPLICABLE CODES AND ORDINANCES. REFER TO CHAPTER 35 OF THIS IBC FOR THE APPLICABLE EDITION OF THE EACH STANDARD
3. THE CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE STRUCTURE SHOWN ON THESE DRAWINGS IS STRUCTURALLY SOUND ONLY IN THE COMPLETED FORM. GENERAL CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, WORKMAN AND OTHER PERSONS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT TO BE LIMITED TO, BRACING, SHORING FOR CONSTRUCTION EQUIPMENT AND SHORING FOR THE STRUCTURE.
4. IN NO CASE SHALL DIMENSIONS BE SCALED FROM DRAWINGS AND/OR DETAILS. ANY DISCREPANCIES FOUND WITHIN THE CONTRACT DOCUMENTS SHALL BE BROUGHT TO THE ARCHITECTS AND THE ENGINEERS ATTENTION FOR CLARIFICATION PRIOR TO PROCEEDING. ANY WORK INSTALLED PRIOR TO AND/OR IN CONFLICT WITH SUCH CLARIFICATION SHALL BE CORRECTED BY THE CONTRACTOR AT HIS EXPENSE AND AT NO ADDITIONAL COST TO THE OWNER.
5. WHERE A DETAIL, SECTION OR A NOTE IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL LIKE OR SIMILAR CONDITIONS UNLESS NOTED OTHERWISE.
6. GENERAL CONTRACTOR TO NOTIFY ARCHITECT AND ENGINEER AS TO ANY DISCREPANCIES BETWEEN SPECIFICATIONS, GENERAL NOTES, STRUCTURAL PLANS AND DETAILS.
7. ANY REFERENCES TO THE WORDS APPROVED, OR APPROVAL IN THESE DOCUMENTS SHALL BE HERE DEFINED TO MEAN GENERAL ACCEPTANCE OR REVIEW AND SHALL NOT RELIEVE THE CONTRACTOR AND/OR HIS SUBCONTRACTORS OF ANY LIABILITY IN FURNISHING THE REQUIRED MATERIALS OR LABOR SPECIFICATION.
8. ALL MATERIALS SHALL BE FURNISHED AS SHOWN HEREIN UNLESS ALTERNATES ARE APPROVED IN WRITING BY THE ARCHITECT, OWNER, AND STRUCTURAL ENGINEER OF THE RECORD.
9. CONNECTIONS OF ALL ITEMS SUPPORTED BY THE STRUCTURE ARE THE RESPONSIBILITY OF THE DISCIPLINES WHO MAKE THESE ATTACHMENTS. REVIEW AND COORDINATE ALL THE REQUIREMENTS AND THE ARCHITECTS PROJECT SPECIFICATIONS AS APPLICABLE.
10. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION AND COORDINATION WITH ARCHITECTURAL, CIVIL, MECHANICAL, ELECTRICAL, PLUMBING AND FIRE SPRINKLER DRAWINGS. THE CONTRACTOR IS RESPONSIBLE FOR THE COORDINATION OF ALL WORK, INCLUDING THAT OF ALL SUBTRADES.
11. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ALL EXISTING UTILITIES WHETHER INDICATED ON CONTRACT DRAWINGS OR NOT, AND TO PROTECT THEM FROM DAMAGE. REPAIR AND REPLACE OF SAID WORK SHALL BE AT THE EXPENSE OF THE CONTRACTOR.
12. UNLESS NOTED, SUBMIT SHOP DRAWINGS FOR ALL FABRICATED MATERIALS. SHOP DRAWINGS WILL NOT BE REVIEWED UNLESS, THEY ARE APPROVED AND ACCEPTED WITH A STAMP FROM GENERAL CONTRACTOR AS CONFORMING TO THE CONSTRUCTION DOCUMENTS.
13. VIBRATIONAL EFFECTS OF MECHANICAL AND/OR ANY OTHER EQUIPMENT HAVE NOT BEEN CONSIDERED BY THE STRUCTURAL ENGINEER.
14. UNLESS NOTED OTHERWISE, ELEVATIONS SHOWN ON THE DRAWINGS ARE TO TOP OF BEAMS AND FOUNDATION.

B. DESIGN CRITERIA

1. CODE: THE 2012 INTERNATIONAL BUILDING CODE
2. ROOF DEAD LOADS = 12 PSF
3. ROOF LIVE LOADS = 20 PSF (REDUCIBLE)
4. ROOF SNOW LOADS = 30 PSF
5. WIND LOADS
   A. BASIC WIND SPEED (3 SECOND GUST) = 115 mph
   B. WIND LOAD IMPORTANCE FACTOR = 1.0
   C. WIND EXPOSURE CATEGORY "C" FOR MAIN WINDFORCE–RESISTING SYSTEM
   D. WIND EXPOSURE CATEGORY "C" FOR COMPONENTS AND CLADDING
6. EARTHQUAKE DESIGN DATA
   SEISMIC IMPORTANCE FACTOR, I = 1.0
   SEISMIC USE GROUP = I
   MAPPED SPECTRAL RESPONSE ACCELERATIONS
   Ss = 0.177g, S1 = 0.060g
   SITE CLASS = D
   SITE COEFFICIENTS
   Fa = 1.6, Fv = 1.5
   CUMULATED SPECTRAL RESPONSE COEFFICIENTS
   Sds = 0.141, Sd1 = 0.068g
   SEISMIC DESIGN CATEGORY = C
   BASIC SEISMIC FORCE RESISTING SYSTEM:
7. ORDINARY REINFORCED MASONRY SHEAR WALLS, ASCE 7–10, TABLE 12.2–1

   SEISMIC RESPONSE COEFFICIENT, Cs = → 0.034
   RESPONSE MODIFICATION FACTOR, R = → 2.0
   SYSTEM OVERSTRENGTH FACTOR Ω = → 2.5
   DEFLECTION AMPLIFICATION FACTOR C_d = → 1.75
   REDUNDANCY FACTOR, p = → 1.0

   ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE ANALYSIS
   DESIGN BASE SHEAR, V = Cs*W
8. FROST LINE DEPTH 30"

C. FOUNDATION

1. FOUNDATION DESIGN IS BASED ON GEOTECHNICAL REPORT BY "TERRACON CONSULTANTS, INC" PROJECT #23145019, DATED JUNE 30, 2014
2. ALL DESIGN PARAMETERS SHALL BE VERIFIED BY THE GEOTECHNICAL ENGINEER OF RECORD. DESIGN SOIL NET BEARING PRESSURE 3000 PSF AT DEPTH OF 30 INCHES BELOW EXISTING GRADE FOR CONTINUOUS FOOTINGS.
3. SITE SHALL BE PREPARED IN COMPLIANCE WITH THE SOILS REPORT, AND WITH COORDINATION OF CONTRACT DOCUMENTS. THE MOST CONSERVATIVE RECOMMENDATIONS SHALL GOVERN.
4. THE GEOTECHNICAL REPORT IN ITS ENTIRETY SHALL BE MADE AS PART OF THE CONTRACT DOCUMENTS.
5. THE CONTRACTOR SHALL INFORM THE ARCHITECT AND ENGINEER OF LOCATION OF EXISTING UTILITIES AND COMMENCE WORK ONLY AFTER WRITTEN APPROVAL FROM THE ARCHITECT.
6. ALL FOUNDATION EXCAVATION, FILLING, BACK FILLING, AND BUILDING PADS SHALL BE INSPECTED AND APPROVED BY THE SOILS ENGINEER.
7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING NECESSARY TO SUPPORT CUT AND/OR FILL SECTIONS DURING EXCAVATION, AND FOR FORMING AND PLACEMENT OF CONCRETE.
8. BACK FILL AGAINST WALLS SHALL BE PLACED EVENLY AGAINST BOTH SIDES OF WALLS UNTIL THE LOWER GRADE IS REACHED.
9. ALL RETAINING WALLS HAVE BEEN DESIGNED AS A FREE DRAINING CONDITION. DESIGN FOR THE DRAINAGE SYSTEM SHALL BE BY OTHERS AND SHALL MEET THE REQUIREMENTS OF THE SOILS REPORT.
10. FOUNDATION WALLS THAT RETAIN EARTH SHALL BE BRACED AGAINST BACKFILLING PRESSURES UNTIL FLOOR SLABS AT TOP AND BOTTOM ARE IN PLACE.

D. CONCRETE

1. ALL CONCRETE MATERIALS, CONSTRUCTION AND WORKMANSHIP SHALL CONFORM TO THE LATEST ADOPTED EDITION OF THE ACI318, ACI306R, INTERNATIONAL BUILDING CODE & PROJECT SPECIFICATIONS.
2. CONCRETE MIXES SHALL BE DESIGNED BY A RECOGNIZED TESTING LABORATORY. USE OF FLYASH IS NOT ALLOWED FOR ANY CONCRETE.
3. CEMENT TYPE SHALL BE IN CONFORMANCE WITH TABLE BELOW AND ASTM C–150, WITH MAXIMUM 8% OF TRICALCIUM ALUMINATE (CA3AL).
4. AGGREGATES FOR NORMAL WEIGHT CONCRETE SHALL CONFORM TO ASTM C–33.
5. MAXIMUM SLUMP AT PLACEMENT SHALL BE 5 INCHES PLUS OR MINUS 1 INCH FOR ALL CONCRETE.
6. UNLESS NOTED THE MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE AS DESIGNATED BELOW;

SULFATE EXPOSURE	WATER SOLUBLE SULFATE (SO_4) IN SOIL, % OF WEIGHT	SULFATE (SO_4) IN WATER PPM	CEMENT RATIO	MAXIMUM WATER/CEMENT RATIO	MIN. CONCRETE STRENGTH f'c, PSI	
					FOUND.	SLAB ON GRADE
NEGLIGIBLE	$0.00 \leq SO_4 < 0.10$	$0 \leq SO_4 < 150$	II	0.55	3500	3500

* MINIMUM MODULES OF RUPTURE 550 PSI.
** MAXIMUM WATER/CEMENT RATIO=0.45
*** POZZOLAN THAT HAS BEEN DETERMINED BY TEST OR SERVICE RECORD TO IMPROVE SULFATE RESISTANCE WHEN USED IN CONCRETE CONTAINING TYPE V CEMENT.

GENERAL CONTRACTOR SHALL PROVIDE MIX DESIGN TO ADDRESS THE SULFATE CHARACTERISTICS OF THE ON–SITE SOIL, THE AGGREGATE BASE (PROVIDED BY THE CONTRACTOR) AND ALL SOILS, SUBBASE, AGGREGATE BASE, ETC. THAT WILL BE IN DIRECT CONTACT WITH THE CONCRETE. THE CONTRACTOR SHALL PROVIDE AN ANALYTICAL REPORT FROM A STATE CERTIFIED LABORATORY OF THE SULFATE CONTENT FOR ALL AGGREGATE BASE AND IMPORT SOILS PRIOR TO BEING DELIVERED TO THE PROJECT SITE AND INCLUDED IN THE BUILDING CONSTRUCTION. AGGREGATE BASE THAT IS IN DIRECT CONTACT WITH THE SLAB–ON–GRADE AND/OR FOUNDATIONS SHALL HAVE A SOLUBLE SULFATE CONTENT LESS THAN 150 PARTS PER MILLION (PPM).

7. CONCRETE CURING SHALL COMPLY WITH ACI 308. FOR FINISH SEE ARCHITECTURAL DRAWINGS.
8. ALL REINFORCING BARS, WIRE MESH, ANCHOR BOLTS, HOLD DOWN ANCHORS AND OTHER CONCRETE INSERTS SHALL BE SECURED IN CORRECT PLACEMENT POSITION, AND APPROVED BY THE BUILDING INSPECTOR PRIOR TO PLACING CONCRETE.
9. UNLESS NOTED, MAXIMUM SPACING OF FLOOR CONTROL JOINTS FOR INTERIOR SLAB–ON–GRADE SHALL BE 15'–0" IN EACH DIRECTION. AS SHOWN ON THE PLANS AND 30 TIMES SLAB THICKNESS FOR EXTERIOR SLAB ON GRADE, SEE PLANS. NO DEVIATIONS FROM THE CONTROL JOINTS SHOWN ON THE PLANS ARE ALLOWED UNLESS APPROVED BY THE PROJECT ARCHITECT. THE CONTRACTOR MAY PLACE CONSTRUCTION JOINTS IN SLAB AS NEEDED IN COMPLIANCE W/HD CRITERIA.
10. PIPES, DUCTS, CONDUITS, ETC. SHALL NOT BE PLACED IN SLABS.
11. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ACCESSORIES, LOCATION OF FLOOR, SLAB DEPRESSIONS, SIDEWALKS AND EXTERIOR SLAB.
12. A QUALIFIED GEOTECHNICAL ENGINEER SHALL CERTIFY THE PAD PRIOR TO SLAB POUR. IF WEATHER CONDITIONS CHANGE, RE–CERTIFICATION IS REQUIRED. SLAB WITH CRACKS WILL NOT BE ACCEPTED. SLAB REPLACEMENT SHALL BE AT OPTION OF OWNER AT CONTRACTOR'S EXPENSE,

E. WELDING

1. ALL WELDING SHALL CONFORM TO THE LATEST EDITION OF THE STRUCTURAL WELDING CODE ANSI/AWS D1.4–04.
2. ALL WELDING SHALL BE PERFORMED USING THE SHIELDED METAL ARC WELDING PROCESS (SMAW) WITH E70XX ELECTRODES OR THE SUBMERGED ARC WELDING PROCESS (SAW) WITH E70XX. LOW HYDROGEN ELECTRODES SHALL BE USED AND PARENT METALS SHALL BE PREHEATED.
3. ALL WELDING SHALL BE PERFORMED BY A CERTIFIED WELDER. ALL FIELD WELDING SHALL BE CONTINUOUSLY INSPECTED BY A REGISTERED DEPUTY INSPECTOR. ALL FIELD WELDING MUST BE INDICATED ON SHOP DRAWINGS.
4. ALL EXPOSED WELDED CONNECTIONS SHALL BE FILLED AND GROUND SMOOTH AND SUBJECT TO ARCHITECTS APPROVAL.
5. ALL WELDS NOT SPECIFIED SHALL BE CONTINUOUS FILLET WELDS. SIZE OF WELDS SHALL BE BASED ON AISC STANDARDS FOR THICKER MATERIAL CONNECTED.

F. REINFORCING STEEL

1. ALL REINFORCING STEEL NO. 4 OR LARGER SHALL CONFORM TO THE REQUIREMENTS OF ASTM A615, GRADE 60 AND NO. 3 OR SMALLER, SHALL CONFORM TO ASTM A615, GRADE 40 U.N.O.. ALL REINFORCING SHALL BEAR MILL STOCK IDENTIFICATION.
2. CLEAR COVERAGE OF CONCRETE OVER OUTER REINFORCING BARS SHALL BE AS FOLLOWS:
   A. CONCRETE CAST AGAINST EARTH 3"
   B. CONCRETE TILT–UP PANELS CAST AGAINST CONC SLAB 1"
   C. CAST–IN–PLACE WALLS;
      NO. 5 & SMALLER 1 1/2"
      NO. 6 & LARGER 2"
3. BARS SHALL BE CLEAN OF RUST, MUD, OIL, GREASE OR OTHER COATING MATERIAL LIKELY TO IMPAIR BONDING.
4. GRADE 60 REINFORCING BARS SHALL NOT BE BENT WITHOUT THE APPROVAL OF THE ENGINEER.
5. ALL REINFORCEMENT MUST BE CONTINUOUS EXCEPT AS DETAILED. ALL SPLICE AND LAP LOCATIONS SHALL BE VERIFIED WITH THE ENGINEER.
6. ALL WELDED REINFORCING STEEL SHALL CONFORM TO ASTM A706, GRADE 60. REINFORCING STEEL WELDING SHALL CONFORM TO AWS D1.4–05. WELDING SHALL BE PERFORMED BY A CERTIFIED WELDER USING LOW HYDROGEN E70XX ELECTRODES. ALL FIELD WELDING SHALL BE CONTINUOUSLY INSPECTED BY A REGISTERED DEPUTY INSPECTOR. NO WELDING SHALL BE DONE AT THE BEND IN A BAR.
7. CONTRACTOR SHALL NOT PLACE ANY REINFORCING UNTIL SHOP DRAWINGS HAVE BEEN APPROVED.
8. WELDED WIRE MESH SHALL CONFORM TO ASTM A 185 GRADE 65 FOR PLAIN WIRE AND ASTM A 497 GRADE 75 FOR DEFORMED BAR.

G. STRUCTURAL STEEL

1. ALL MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE LATEST EDITION OF AISC MANUAL OF STEEL CONSTRUCTION AND SHALL BE DETAILED, FABRICATED AND ERECTED IN CONFORMANCE WITH THE AISC SPECIFICATIONS. LICENSED FABRICATOR IS REQUIRED FOR STRUCTURAL STEEL FABRICATION.
2. MATERIALS:
   - TUBE MEMBERS — ASTM A500, GRADE B (F_Y=46 KSI)
   - PIPE COLUMNS — ASTM A53, GRADE B
   - SHAPES COLUMNS & BEAMS — ASTM A992 GRADE 50
   - ANGLES, CHANNELS, PLATES & BARS — ASTM A36
   - HIGH STRENGTH BOLTS — ASTM 325N OR A490
   - ERECTION BOLTS — ASTM A307, GRADE "B"
   - SHEAR STUDS — ASTM A108 & A.W.S. D1.1 (F_Y=65KSI)
   - HEAVY HEX HEADED ANCHOR BOLTS — ASTM F1554, GRADE 36
   - WASHERS — ASTM F436
   - NUTS — ASTM A563
   - THREADED RODS — ASTM A36
   - HANGER RODS — ASTM F1554, GRADE 36
3. STRUCTURAL STEEL FABRICATORS SHALL FURNISH SHOP DRAWINGS OF ALL STEEL FOR REVIEW OF STRUCTURAL ENGINEER PRIOR TO FABRICATION.
4. STEEL FABRICATOR MUST BE REGISTERED AND BE APPROVED BY THE CITY DEPARTMENT OF BUILDING AND SAFETY FOR THE FABRICATION OF MEMBERS AND ASSEMBLIES ON THE PREMISES OF THE FABRICATOR'S SHOP.
5. EXCEPT WHERE ENCASED IN MASONRY, CONCRETE OR SPRAYED ON FIRE PROOFING. ALL STEEL SHALL RECEIVE ONE SHOP COAT OF RUST INHIBITIVE PRIMER.
6. HOLES SHALL NOT BE PLACED IN STEEL MEMBERS UNLESS SPECIFICALLY DETAILED ON DRAWINGS. STEEL MEMBERS SHALL BE SHORED WHEN PERMISSIBLE HOLES ARE CUT OR BURNED. BOLT HOLES SHALL CONFORM TO AISC SPECIFICATION, AND SHALL BE STANDARD HOLES UNLESS OTHERWISE NOTED.
7. ALL EXPOSED ENDS OF STEEL MEMBERS SHALL BE CAPPED WITH STEEL PLATE. 1/4" IN THICKNESS WITH GROUND SMOOTH WELD.
8. GROUTING OF BASE PLATES SHALL BE WITH NON–SHRINK GROUT OR DRYPACKED WITH MINIMUM COMPRESSIVE STRENGTH AS SPECIFIED IN THE CONCRETE SECTION OF NOTES. ALL SURFACES SHALL BE CLEAN OF FOREIGN MATERIAL PRIOR TO GROUTING.
9. HIGH STRENGTH BOLTS SHALL CONFORM TO ASTM A325N OR A490, AND BE PROVIDED WITH HARDENED WASHERS CONFORMING TO ASTM F436.
10. STRUCTURAL STEEL SHALL BE DELIVERED TO THE JOB SITE FREE OF RUST, MILL SCALE, GREASE, ETC.
11. THE ROLLED STEEL SECTIONS AND BOLTS MANUFACTURES OUTSIDE THE UNITED STATES WILL REQUIRE VERIFICATION THAT THE PRODUCTS COMPLY WITH APPLICABLE ASTM STANDARDS. MILL CERTIFICATES WILL BE REQUIRED FOR ALL STEEL. STEEL GRADES OTHER THAN ASTM A36 WILL REQUIRE TESTING BY AN APPROVED LABRORATORY. ALL FOREIGN BOLTS MUST BE APPROVED BY CITY DEPARTMENT OF BUILDING AND SAFETY PRIOR TO THEIR USE.

H. METAL DECK

ROOF DECK SHALL BE GALVANIZED OR PAINTED TYPE "BW" (WIDE RIB) THREE SPAN MIN AS SHOWN ON ROOF FRAMING PLAN. THE STEEL ROOF DECK IS REQUIRED TO ACT AS A DIAPHRAGM. REFER TO THE ROOF DIAPHRAGM CONNECTION DIAGRAM AND SCHEDULE FOR ATTACHMENT PATTERN OPTIONS. PROVIDE 2 LAYERS OF ROOF DECK WHERE SINGLE SPAN CONDITION EXISTS. MAKE DIAPHRAGM CONNECTIONS AFTER PLACEMENT OF BOTH LAYERS OF ROOF DECK.

I. MASONRY

1. CONCRETE MASONRY UNITS (HOLLOW UNITS) SHALL BE GRADE "N" TYPE I, MEDIUM WEIGHT, (MAXIMUM DENSITY 115 PCF) CONFORMING TO ASTM C90 AND CBC STANDARD, MINIMUM ULTIMATE STRENGTH OF BLOCK SHALL BE 1900 PSI. SEE ARCHITECTURAL FOR TYPE, PATTERN AND JOINT DETAILS. ALL UNITS BELOW ROUGH GRADE OR IN CONTACT WITH SOIL SHALL BE NORMAL WEIGHT.
2. GROUT SHALL BE COARSE GROUT (PEA GRAVEL) WITH COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS MAXIMUM DENSITY SHALL BE 140 PCF. GROUT WITHIN 1 1/2 HOURS FROM INTRODUCING WATER INTO THE MIXTURE. ANY ADMIXTURES CONTAINING CHLORIDES SHALL NOT BE USED IN MORTAR OR IN GROUT
3. MORTAR SHALL BE 1800 PSI IN 28 DAYS.TYPE "S" AS DEFINED IN TABLE 21A–A OF THE INTERNATIONAL BUILDING CODE AND CONFORMING TO ASTM C270
4. MINIMUM SPECIFIED STRENGTH OF THE MASONRY ASSEMBLAGE "f'm" SHALL BE 1500 PSI.
5. UNLESS NOTED, LAY MASONRY IN RUNNING BOND. USE CLOSED END UNITS AT CORNERS.
6. GROUTING PROCEDURE FOR LIFTS EXCEEDING FOUR FEET: PROVIDE CLEAN OUT OPENINGS AT THE BOTTOM OF THE LIFT IN EVERY CELL TO BE FILLED. LIFT NOT TO EXCEED 6'–0".
7. ALL CELLS WITH REINFORCEMENT SHALL BE GROUTED SOLID. HOLD TOP OF EACH GROUT 2" DOWN BELOW TOP OF BLOCK.
8. CONTINUOUS INSPECTION OF WORK INVOLVING MASONRY IS REQUIRED, UNLESS NOTED OTHERWISE.
9. PROVIDE OPEN END BLOCKS FOR CONTINUOUS VERTICAL BARS.
10. CONTRACTOR SHALL ADEQUATELY BRACE ALL MASONRY WALLS DURING ERECTION AND NOT REMOVE THESE MEANS UNTIL DESIGN LATERAL SUPPORT IS ADEQUATELY IN PLACE
11. PROVIDE 2–#5 VERTICAL BARS AT ALL ENDS, CORNERS, JAMBS AND INTERSECTIONS OF ALL MASONRY WALLS UNLESS NOTED OTHERWISE HORIZONTAL BEAMS, BOND BEAMS AND REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS
12. PROVIDE MASONRY PRISM TESTING IN ACCORDANCE WITH IBC STANDARDS.
13. CONCRETE MASONRY UNITS SHALL NOT BE WETTED UNLESS SPECIFICALLY APPROVED.
14. UNLESS NOTED, VERTICAL AND HORIZONTAL REINFORCING, TIES AND EMBEDDED ITEMS SHALL BE IN PLACE PRIOR TO GROUTING. UNLESS NOTED, VERTICAL AND HORIZONTAL REINFORCING LAP SPLICE SHALL BE 48 BAR DIAMETERS, BUT NOT LESS THAN 12" USE POSITIONERS TO HOLD VERTICAL REINFORCING IN PLACE. SPACING OF POSITIONERS SHALL NOT EXCEED 200 BAR DIAMETERS NOR 10 FEET LOCATE 8" MAX. ABOVE TOP OF GROUT LEFT, TYPICALLY

J. LIGHTGAGE METAL FRAMING

1. AN APPLICATION TO PERFORM OFF–SITE COLD FORM STEEL FABRICATION IS REQUIRED AND MUST BE SUBMITTED TO THE INSPECTION SERVICES DIVISION FOR APPROVAL PRIOR TO FABRICATION.
2. ALL LIGHTGAGE METAL FRAMING NOT SPECIFIED BY SIZE, GAGE AND CONFIGURATION ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR. CONNECTIONS OF ALL LIGHTGAGE METAL FRAMING MEMBERS (MEMBER TO MEMBER AND MEMBER TO PRIMARY STRUCTURE) NOT SHOWN ON THE DRAWINGS SHALL BE DESIGNED BY THE CONTRACTOR.
3. DESIGN SHALL BE IN ACCORDANCE WITH ALL APPLICABLE CODES, INCLUDING: IBC, AISC, AISI AND AWS.
4. LATERAL LOAD CRITERIA – SEE SECTION "B"
5. DESIGN SHALL ALLOW FOR LATERAL DEFLECTIONS DUE TO WIND OR SEISMIC FORCES PER IBC.
6. CONTRACTOR SHALL COORDINATE THE CONNECTION OF WINDOW SYSTEMS AND OTHER ARCHITECTURAL ITEMS TO LIGHTGAGE METAL FRAMING MEMBERS. LOADS FROM THE FRAME SHALL BE INCLUDED IN THE DESIGN OF THE LIGHTGAGE METAL FRAMING MEMBERS.
7. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS SIGNED BY A REGISTERED CIVIL OR STRUCTURAL ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT OCCURS, FOR REVIEW BY THE BUILDING OFFICIAL AND THE ARCHITECT FOR CONDITIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
8. SIZES OF LIGHTWEIGHT METAL FRAMING SHALL COMPLY WITH THE STRUCTURAL AND ARCHITECTURAL DRAWINGS.
9. EXTERIOR LIGHTGAGE METAL FRAMING SHALL HAVE A G–60 GALVANIZED COATING PER ASTM 525. ALL OTHER LIGHTGAGE METAL FRAMING SHALL BE PRIMED WITH RUST INHIBITIVE PAINT MEETING THE PERFORMANCE REQUIREMENTS OF TT–P–636 C.
10. LIGHTGAGE METAL FRAMING CALLED OUT ON THE STRUCTURAL DRAWINGS BY TYPE, SIZE AND GAGE SHALL BE FURNISHED BY THE STEEL STUD MANUFACTURER'S ASSOCIATION (ICC–ES REPORT ESR–3064P)
11. ALL STUD / TRACK MATERIAL TO CONFORM TO THE FOLLOWING:
12. 16GA AND HEAVIER – 50 KSI MINIMUM YIELD
    PAINTED STEEL PER ASTM A570 – GRADE 50
    GALVANIZED STEEL PER ASTM A446 – GRADE D
13. 18GA AND LIGHTER – 33 KSI MINIMUM YIELD
    PAINTED STEEL PER ASTM A570 – GRADE 33
    GALVANIZED STEEL PER ASTM A446 – GRADE A
14. FASTENERS FOR CONNECTING LIGHTGAGE METAL FRAMING MEMBERS SHALL BE #10x5/8" PANHEAD SELF DRILLING AND TAPPING SHEET METAL SCREWS U.N.O. ON THE STRUCTURAL DRAWINGS. SCREWS SHALL HAVE A WORKING STRESS CAPACITY OF 240# SHEAR AND 90# TENSION IN 18 GA MATERIAL.
15. ALL LIGHT GAGE METAL FRAMING AS PER SSMA(STEEL STUD MANUFACTURERS ASSOCIATION) ICC–ES REPORT ESR–3064P. WEB SITE WWW.SSMA.COM.

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PROFESSIONAL IN CHARGE
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PROJECT MANAGER
R NEW
QUALITY CONTROL
R NEW
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FA

PROJECT NAME
CIRCLE K STORES, INC. CAR WASH
COLORADO SPRINGS COLORADO
E. FOUNTAIN BLVD & ACADEMY PARK LOOP

CIRCLE K STORES INC.

PROJECT NUMBER
2014–066
SHEET TITLE
GENERAL NOTES

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